

SSC-227

**TANKER TRANSVERSE STRENGTH ANALYSIS
USER'S MANUAL**

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SHIP STRUCTURE COMMITTEE

1972

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SECRETARY
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U.S. COAST GUARD HEADQUARTERS
WASHINGTON, D.C. 20591

SR-196
1972

Dear Sir:

One of the most important goals of the Ship Structure Committee is the improvement of methods for design and analysis of ship hull structures. This report is the third in a sequence of four Ship Structure Committee reports on a project directed toward development of an accurate, but less expensive, computer aided structural analysis method.

This report contains the User's Manual for the transverse strength analysis portion of the program. Other reports of this project are:

- SSC-225 - Structural Analysis of Longitudinally Framed Ships
- SSC-226 - Tanker Longitudinal Strength Analysis-- User's Manual and Computer Program
- SSC-228 - Tanker Transverse Strength Analysis-- Programmer's Manual

Comments on this report would be welcomed.

Sincerely,



W. F. REA, III
Rear Admiral, U. S. Coast Guard
Chairman, Ship Structure Committee

SSC-227

Final Report

on

Project SR-196, "Computer Design of
Longitudinally Framed Ships"

to the

Ship Structure Committee

TANKER TRANSVERSE STRENGTH ANALYSIS

USER'S MANUAL

by

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COM/CODE Corporation

under

Department of the Navy
Naval Ship Engineering Center
Contract No. N00024-70-C-5219

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U. S. Coast Guard Headquarters
Washington, D. C.
1972

ABSTRACT

This report, the third in a sequence of four Ship Structure Committee Reports on a method for performing structural analysis of a tanker hull, contains the User's Manual for the transverse strength analysis portion of the program.

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INTRODUCTION:

This user's manual is applicable to the transverse analysis program only. What follows defines all necessary input parameters and the formats in which they are to be entered into the computer. No attempt has been made to instruct the user in the optimum modeling techniques for a given analysis study.

PROGRAM DESCRIPTION:

The transverse strength analysis for longitudinally framed ships combines the techniques of finite elements with a newly developed method of uncoupling the three dimensional structure so as to reduce computational time and to permit a finer mesh analysis without the usual resulting degradation in numerical precision.

Transverse members are modeled with appropriate quadrilateral plate (linearly varying stress) elements and bar (axially elastic) stiffeners. Near the edges of the transverse openings, triangular plate (constant stress) elements may also be generated. All longitudinal members spanning transverses are represented by by-planar beam elements which carry all external loads directly onto the transverses. Shear loads upon the transverse members as developed by the side shell and webbing of the longitudinal bulkhead are treated as additional loading functions upon the ship's structure.

Most finite elements are generated automatically by the program. Various other convenience features such as printer simulated plots of the transverse finite element definition have been implemented.

Output from the analysis include both plate and bar stresses within the transverse member's structure.

While the present analysis capability is limited to loading conditions which are symmetric about the hull centerline, the unsymmetric cases may be analyzed by manual superposition of the antisymmetric component solutions.

GENERAL GUIDELINES:

1. Each data card (except for appropriate system control cards) must begin with the proper label. The label must begin in Column 1 exactly as given in the data sheets that follow. A label must not exceed 10 characters in length.
2. No blank cards are permitted between data cards.
3. All numerical information must be entered per the appropriate format. All field lengths for numerical data are 10 columns.
4. All numerical data must be given with a decimal point. No data is presumed to be integer.
5. The program initiates a large number of data checks, primarily with regard to the order of the input cards. Any error detected by the program will cause a premature program termination at the conclusion of the given data input subject to where the error was found.
6. It is recommended that the user allow the program to make at least one complete pass through the entire data card deck before a complete stress solution is attempted. The user may make such a preliminary pass by omitting the EIGENS card which normally follows the last data card defining the loading condition upon the structure. See sample execution times.

GENERAL LOGIC DIAGRAM FOR TRANSVERSE ANALYSIS

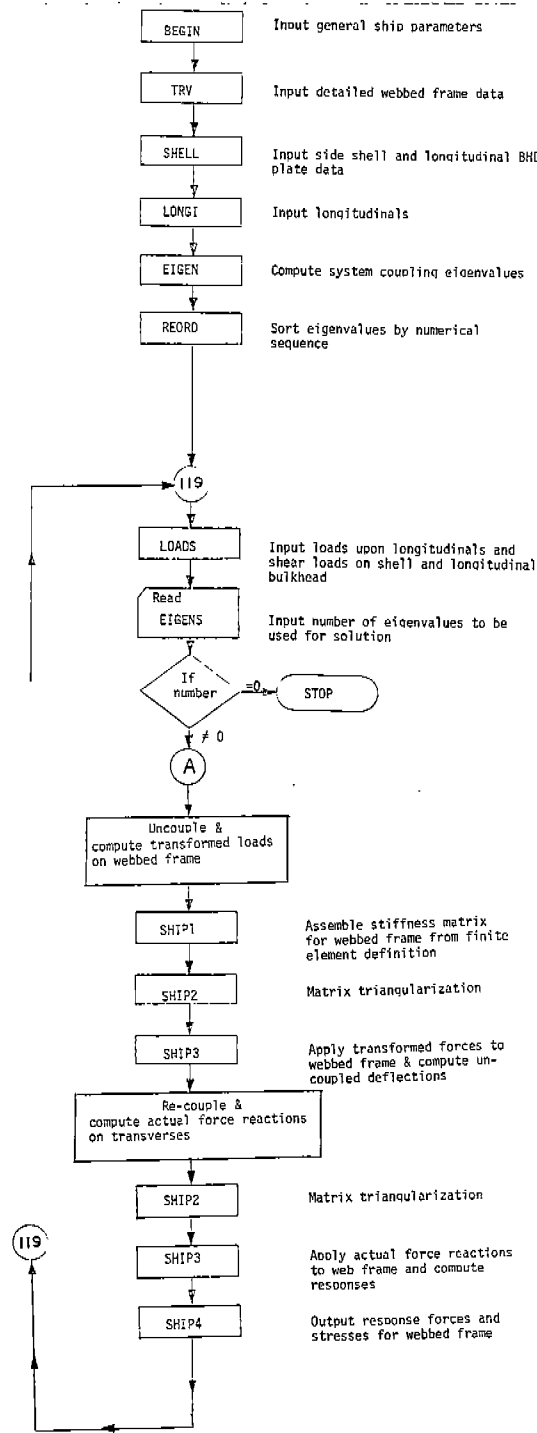


Fig. 1a. General Logic Diagram For Transverse Analysis

1	5	10	15	20	25	30
FACTOR						

Conversion factor to be multiplied by the program to all units of length of all data items that have a length dimension. This includes Young's modulus and distributed loads.

	force	length
UNITS		

Alphanumeric labels for the output force units (cc 11-20) and the output length units after conversion (cc 21-30)

LENGTH	
--------	--

Length of ship's section under analysis

51

	E	μ
MATERIAL		

Material properties: E(cc 11-20), and Poisson's (cc 21-30)

SPACING	
---------	--

Spacing for transverse members (assumed equal)

NOTE: a given ship's section can have no more than 50 transverse members (not including the ends)

STIFFNESS is a measure of each transverse member's relative stiffness when modeled as a shear beam. Normally, the web frame is assigned a nominal stiffness of 1.0. The more substantial members such as the oil-tight bulkhead, then, would be given an appropriately higher value. Enter these relative stiffness ratios in position order of transverses starting with the aft-most member in the ship section being included in the analysis. Use only enough STIFFNESS cards to complete the stiffness definition of all transverses involved (maximum = 50).

1	5	10	15	20	25	30	35	40	45	50	55	60	65	70
STIFFNESS														
STIFFNESS														
STIFFNESS														
STIFFNESS														
STIFFNESS														
STIFFNESS														
STIFFNESS														
STIFFNESS														

5

No END card is needed in this section. The computer will be looking for a total of $(\text{LENGTH}/\text{SPACING}) + 1$ ratios to be entered. Do not enter blank ratios within the list (from card columns 11 through 70 per card) unless the list is exhausted on the last STIFFNESS card.

The ANALYZE card permits the user to select up to 5 different transverse members within the ship's section for detailed analysis of the stresses. Enter those transverses by their relative position from the aft end of the section.

ANALYZE

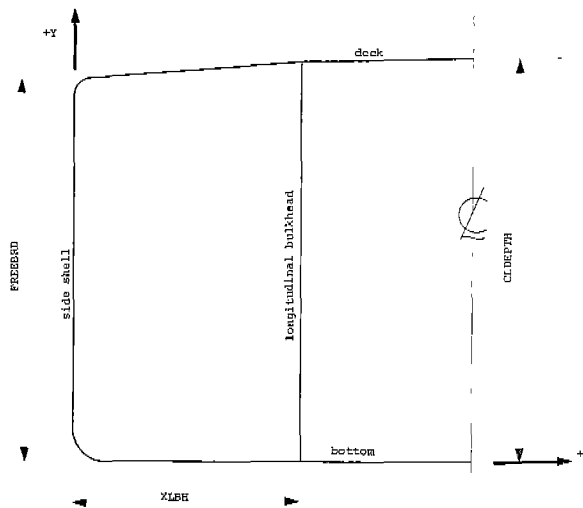
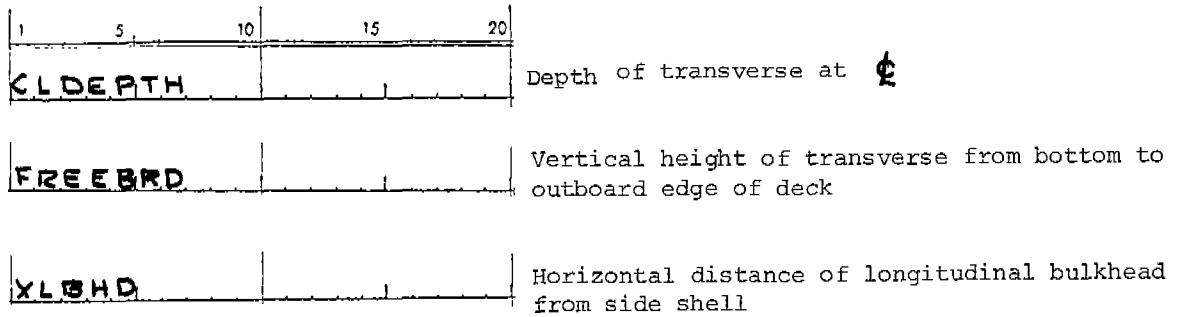


Fig. 2. General Parameters for a Transverse Section

FINITE ELEMENT DEFINITION OF THE TRANSVERSES SUBMITTED FOR ANALYSIS:

The user will create a rectangular grid system for the automatic generation of the plate and bar finite elements comprising the given transverse member. This pattern must be developed from horizontal/vertical rows and columns, which normally fit quite well the pattern arrangement for tanker longitudinals within the midbody section. A finer element mesh is possible by including extra rows and columns. Not all longitudinals need coincide with the row/column intersections (called nodes), but those that do not should reflect this condition by adjusting their moments of inertia. It is also possible to lump longitudinals at a given node, although

some loss in accuracy should be expected particularly near the area of such modeling approximation.

The following figure illustrates a sample web frame that has been defined within the row/column network.

All nodes are assumed to be the joining points for all plate and bar elements. All longitudinals are assumed to pass through appropriate nodes.

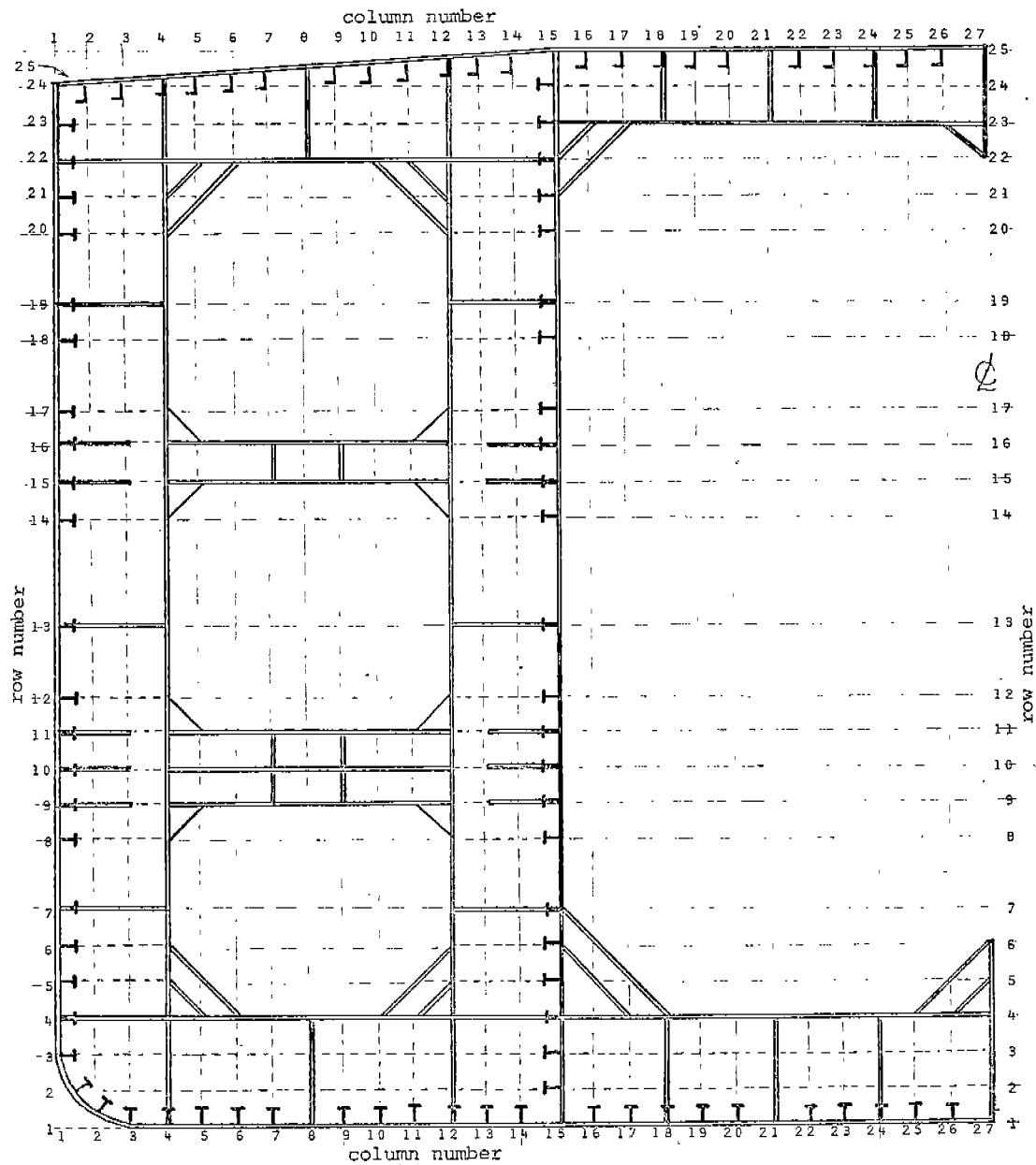
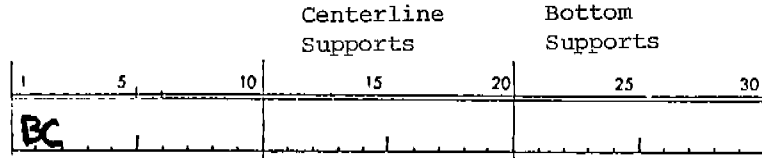


Fig. 3. Finite Element Grid Definition for Transverse Frame

Boundary Conditions:

The BC card (for Boundary Conditions) defines what direction deflections are presumed restricted along the hull center line and at the support assumed located at the bottom longitudinal beneath the longitudinal bulkhead. As input to this card, a special code is used:

- 1.0 means restricted x-deflections (normally at C.L.)
- 0.0 means restricted y-deflections (normally at bottom)



The WEIGHT card prescribes the weighting factor applied to the boundary supports, which are not treated as indeterminants. Normally a weighting factor of 1.0 is used; a larger number holds the support more firmly, although care should be taken not to use too high a value since numerical problems may result.

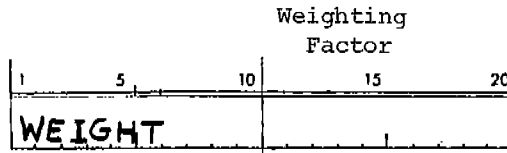


PLATE cards define various plate thickness areas within the grid pattern. Plate so defined covering VOID areas, however, will automatically be ignored.

Different plate thicknesses should not be defined for the same area of the transverse section since only the first thickness will be used.

The order given for the start and end rows and columns is quite arbitrary. These coordinates merely provide the edge boundaries for the given plate thickness.

Use as many PLATE cards as needed (maximum = 50).

An END card is required to follow.

BAR cards define various bar element cross-sectional areas for all stiffeners, face plates, and flanges normal to the transverse plane. Included in this list of elements should be portions of deck, bottom, side shell, and longitudinal bulkhead plate material which contribute to stiffening adjacent transverse plating.

Different bar areas should not be defined for the same node spans since only the first area entered in the input stream will be used.

A given bar may span any number of nodes as long as the nodes are all in the same row, or all in the same column, or on a 1:1 diagonal within the grid network.

Since bars are uni-directional, care must be taken in specifying the start and end rows and columns. The start row must coincide with the same node as the start column; the same applies for the end node.

Use as many BAR cards as needed (maximum = 100).

An END card is required to follow the last BAR card.

DEFINITION OF LONGITUDINALS:

STANDARD defines the moment of inertia and cross-sectional area of the standard or typical longitudinal within the ship structure. Only for this particular longitudinal will influence coefficients be computed. With usually very little loss of solution accuracy, the corresponding influence coefficients for the remaining longitudinals are obtained by a simple stiffness proportioning. This procedure greatly reduces overall computational time during the analysis uncoupling.

			moment of inertia			area		
1	5	10	15	20	25	30		
STANDARD								

The following data sheets define all longitudinal members to be modeled. Use as many appropriate cards as needed. Note that two types of cards may be used: XLONG and YLONG. If a number of longitudinals are identical along a given row and occur in sequence, use XLONG. If a number of longitudinals are identical along a given column and occur in sequence, use YLONG. Either XLONG or YLONG may be used for longitudinals which follow no such sequence but must be defined independently.

There is no required order to the use of XLONG and YLONG cards. However, an END card must follow the last of this set.

DEFINITION OF LOADING CONDITIONS:

All loads acting upon the ship section are assumed to be acting upon only the longitudinals. Along the length of a longitudinal, loads may be almost any combination of concentrated and uniformly distributed forces which act at any arbitrary location on the member. All forces are defined as either X or Y components relative to the global coordinate system prescribed for the transverses.

The general order of loading input is as follows:

1. Define all loading forces of a common direction sense, both concentrated and/or uniform, as applied to a given longitudinal.
2. List all longitudinals so loaded. If there are longitudinals which have similar loads that differ only by a proportional factor, these too may be entered in the list.
3. Repeat steps 1. and 2. above for each set of longitudinal loadings.
4. END card.

The following are the available load cards:

XFORCE, concentrated force acting in the X-direction
 YFORCE, concentrated force acting in the Y-direction
 XUNIFORM, uniformly distributed load acting in X-direction
 YUNIFORM, UNIFORMLY distributed load acting in Y-direction

Note that under step 1 above, X and Y force components cannot be mixed, but must be entered separately.

The listing of longitudinals under a given set of loading forces is done via XLONG and YLONG cards. Note that these cards are somewhat different from those of the same label used to define the longitudinals. However, their use here for loadings is much the same idea as where strings of longitudinals in a horizontal (XLONG) or vertical (YLONG) sequence may be listed in one statement. These cards also provide for proportional factors which will be applied to the load magnitudes directly. Note that if this factor is left blank, or zero, the program will assume a factor of 1.0.

An END card is required after the last XLONG or YLONG which concludes the last loading set.

The following prescribes the formats for each of the loading cards. The loads are given in sub-sets of location (or starting location) from the aft-end of the longitudinal and the corresponding signed magnitude. All loads listed in this manner per card should be entered in location order; the program will ignore all blank or zero location entries past the first one entered on a given card. Note that the uniform loads are listed by their start location; their end location is assumed to be either the forward end of the longitudinal or the start location for the next uniform load.

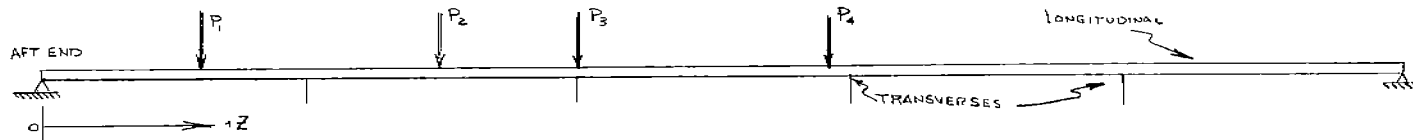
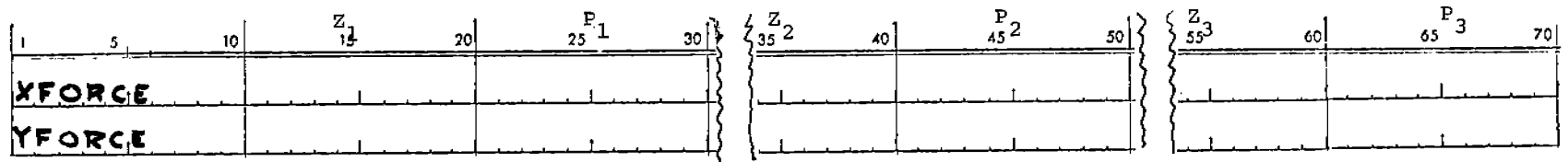


Fig. 4. Concentrated Forces on Longitudinals

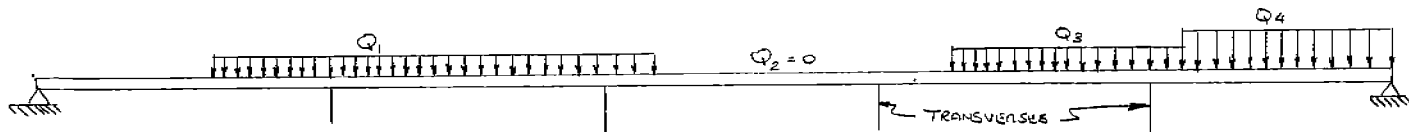
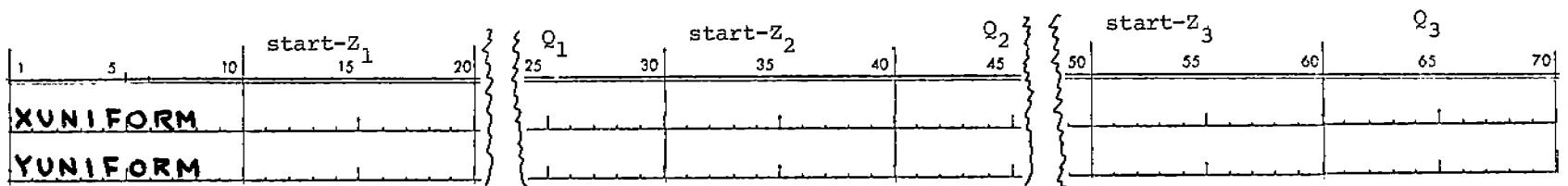


Fig. 5. Uniformly Distributed Loads on Longitudinals

The following prescribes the formats for each of the XLONG and YLONG cards that define what longitudinals specifically pertain to the given loading subset.

	row	start column	end column	factor *						
1	5	10	15	20	25	30	35	40	45	50
XLONG										

	column	start row	end row	factor *						
1	5	10	15	20	25	30	35	40	45	50
YLONG										

A given longitudinal (or sets of identically or similarly loaded longitudinals) may have any number of X and Y loads applied. However, for any given load set, all forces must be of the same component direction; the complementary set of loads will have to follow with these longitudinals re-stated with the appropriate XLONG and YLONG cards. Also, within a given component load set, the user is limited to 50 concentrated forces and 20 uniform loads, which are entered with the appropriate number of X(Y)FORCE and X(Y)UNIFORM cards. If still more loads exist, they must be entered as a separate load set with the longitudinals re-stated.

* The proportional factor permits the given loading subset condition to be adjusted by a constant proportion for the given set of longitudinals. If the

EIGENS is the last card for the complete loading condition definition, after all load sets have been entered with their appropriate X(Y)LONG CARDS. The EIGEN card permits the user to employ some judgment in selecting the number of EIGEN modes which should be necessary to provide adequate solutions. Theoretically, there are an equal number of EIGEN modes as there are transverse members in the ship section. However, it is often not necessary to employ all of these modes within the solution process, particularly if the loading condition is not too irregular along the length of the section. For example, if the overall loading pattern is uniform over the entire section, only the first mode should be necessary. If the load pattern changes direction only once along the length of the section, probably just two modes will suffice. However, if the loading pattern is considerably irregular in that it changes directional sign frequently along the length of the section, it may be best to use all modes (= number of transverses).

1	5	10	15	20
EIGENS				

NOTE: All input should be checked first by the transverse analysis program before a complete solution is attempted. Omitting the EIGENS card will cause the program to terminate prior to the execution of the larger part of the stress solution processing. See sample execution times.

SAMPLE PROBLEM

The following pages offer a sample of the input phase of the program. Portions of the output have been omitted for the purpose of clarifying the input data. The webbed frame of this sample problem is illustrated in Figure 3.

TRANSVERSE STRENGTH ANALYSIS OF LONGITUDINALLY FRAMED SHIPS
BY COM/CODE CORPORATION

CONVERSION FACTOR IS APPLIED TO ALL DIMENSIONAL UNITS OF LENGTH
FACTOR 1.000
UNITS K G C M

LENGTH OF SHIP SECTION TO BE ANALYZED
LENGTH 15402.000

YOUNGS MODULUS, POISSONS RATIO
MATERI .205E+04 .300

SPACING BETWEEN TRANSVERSES
SPACIN 513.400

STIFFNESS FACTORS OF ALL TRANSVERSES IN ORDER FROM STERN

STIFFN	1.000	1.000	2.460	1.000	1.000	5.600
STIFFN	1.000	1.000	2.460	1.000	1.000	5.600
STIFFN	1.000	1.000	2.460	1.000	1.000	2.460
STIFFN	1.000	1.000	5.600	1.000	1.000	2.460
STIFFN	1.000	1.000	2.460	1.000	1.000	

LIST TRANSVERSE BY POSITION FROM STERN THAT ARE TO BE ANALYZED
ANALYZ 16.000

FINITE ELEMENT DEFINITION OF TRANSVERSE

CLDEPT	2565.000	XCOORD	2344.000
FREFBR	2450.000	XCOORD	2438.000
XLBHD	1310.000	YCOORD	0.000
XCOORD	0.000	YCOORD	90.000
XCOORD	89.000	YCOORD	180.000
XCOORD	182.000	YCOORD	264.000
XCOORD	276.000	YCOORD	348.000
XCOORD	370.000	YCOORD	432.000
XCOORD	464.000	YCOORD	516.000
XCOORD	558.000	YCOORD	600.000
XCOORD	652.000	YCOORD	684.000
XCOORD	746.000	YCOORD	768.000
XCOORD	840.000	YCOORD	852.000
XCOORD	934.000	YCOORD	936.000
XCOORD	1028.000	YCOORD	1020.000
XCOORD	1122.000	YCOORD	1104.000
XCOORD	1216.000	YCOORD	1188.000
XCOORD	1310.000	YCOORD	1272.000
XCOORD	1404.000	YCOORD	1356.000
XCOORD	1498.000	YCOORD	1440.000
XCOORD	1592.000	YCOORD	1524.000
XCOORD	1686.000	YCOORD	1608.000
XCOORD	1780.000	YCOORD	1692.000
XCOORD	1874.000	YCOORD	1776.000
XCOORD	1968.000	YCOORD	1860.000
XCOORD	2062.000	YCOORD	1944.000
XCOORD	2156.000	YCOORD	2028.000
XCOORD	2250.000	YCOORD	2112.000
XCOORD	2344.000	YCOORD	2196.000
XCOORD	2438.000	YCOORD	2280.000
XCOORD	2532.000	YCOORD	2364.000
XCOORD	2626.000	YCOORD	2448.000
XCOORD	2720.000	YCOORD	2532.000
XCOORD	2814.000	YCOORD	2616.000
XCOORD	2908.000	YCOORD	2700.000
XCOORD	3002.000	YCOORD	2784.000
XCOORD	3096.000	YCOORD	2868.000
XCOORD	3190.000	YCOORD	2952.000
XCOORD	3284.000	YCOORD	3036.000
XCOORD	3378.000	YCOORD	3120.000
XCOORD	3472.000	YCOORD	3204.000
XCOORD	3566.000	YCOORD	3288.000
XCOORD	3660.000	YCOORD	3372.000
XCOORD	3754.000	YCOORD	3456.000
XCOORD	3848.000	YCOORD	3540.000
XCOORD	3942.000	YCOORD	3624.000
XCOORD	4036.000	YCOORD	3708.000
XCOORD	4130.000	YCOORD	3792.000
XCOORD	4224.000	YCOORD	3876.000
XCOORD	4318.000	YCOORD	3960.000
XCOORD	4412.000	YCOORD	4044.000
XCOORD	4506.000	YCOORD	4128.000
XCOORD	4600.000	YCOORD	4212.000
XCOORD	4694.000	YCOORD	4296.000
XCOORD	4788.000	YCOORD	4380.000
XCOORD	4882.000	YCOORD	4464.000
XCOORD	4976.000	YCOORD	4548.000
XCOORD	5070.000	YCOORD	4632.000
XCOORD	5164.000	YCOORD	4716.000
XCOORD	5258.000	YCOORD	4800.000
XCOORD	5352.000	YCOORD	4884.000
XCOORD	5446.000	YCOORD	4968.000
XCOORD	5540.000	YCOORD	5052.000
XCOORD	5634.000	YCOORD	5136.000
XCOORD	5728.000	YCOORD	5220.000
XCOORD	5822.000	YCOORD	5304.000
XCOORD	5916.000	YCOORD	5388.000
XCOORD	6010.000	YCOORD	5472.000
XCOORD	6104.000	YCOORD	5556.000
XCOORD	6198.000	YCOORD	5640.000
XCOORD	6292.000	YCOORD	5724.000
XCOORD	6386.000	YCOORD	5808.000
XCOORD	6480.000	YCOORD	5892.000
XCOORD	6574.000	YCOORD	5976.000
XCOORD	6668.000	YCOORD	6060.000
XCOORD	6762.000	YCOORD	6144.000
XCOORD	6856.000	YCOORD	6228.000
XCOORD	6950.000	YCOORD	6312.000
XCOORD	7044.000	YCOORD	6396.000
XCOORD	7138.000	YCOORD	6480.000
XCOORD	7232.000	YCOORD	6564.000
XCOORD	7326.000	YCOORD	6648.000
XCOORD	7420.000	YCOORD	6732.000
XCOORD	7514.000	YCOORD	6816.000
XCOORD	7608.000	YCOORD	6900.000
XCOORD	7702.000	YCOORD	6984.000
XCOORD	7796.000	YCOORD	7068.000
XCOORD	7890.000	YCOORD	7152.000
XCOORD	7984.000	YCOORD	7236.000
XCOORD	8078.000	YCOORD	7320.000
XCOORD	8172.000	YCOORD	7404.000
XCOORD	8266.000	YCOORD	7488.000
XCOORD	8360.000	YCOORD	7572.000
XCOORD	8454.000	YCOORD	7656.000
XCOORD	8548.000	YCOORD	7740.000
XCOORD	8642.000	YCOORD	7824.000
XCOORD	8736.000	YCOORD	7908.000
XCOORD	8830.000	YCOORD	7992.000
XCOORD	8924.000	YCOORD	8076.000
XCOORD	9018.000	YCOORD	8160.000
XCOORD	9112.000	YCOORD	8244.000
XCOORD	9206.000	YCOORD	8328.000
XCOORD	9300.000	YCOORD	8412.000
XCOORD	9394.000	YCOORD	8496.000
XCOORD	9488.000	YCOORD	8580.000
XCOORD	9582.000	YCOORD	8664.000
XCOORD	9676.000	YCOORD	8748.000
XCOORD	9770.000	YCOORD	8832.000
XCOORD	9864.000	YCOORD	8916.000
XCOORD	9958.000	YCOORD	9000.000
XCOORD	10052.000	YCOORD	9084.000
XCOORD	10146.000	YCOORD	9168.000
XCOORD	10240.000	YCOORD	9252.000
XCOORD	10334.000	YCOORD	9336.000
XCOORD	10428.000	YCOORD	9420.000
XCOORD	10522.000	YCOORD	9504.000
XCOORD	10616.000	YCOORD	9588.000
XCOORD	10710.000	YCOORD	9672.000
XCOORD	10804.000	YCOORD	9756.000
XCOORD	10898.000	YCOORD	9840.000
XCOORD	10992.000	YCOORD	9924.000
XCOORD	11086.000	YCOORD	10008.000
XCOORD	11180.000	YCOORD	10092.000
XCOORD	11274.000	YCOORD	10176.000
XCOORD	11368.000	YCOORD	10260.000
XCOORD	11462.000	YCOORD	10344.000
XCOORD	11556.000	YCOORD	10428.000
XCOORD	11650.000	YCOORD	10512.000
XCOORD	11744.000	YCOORD	10596.000
XCOORD	11838.000	YCOORD	10680.000
XCOORD	11932.000	YCOORD	10764.000
XCOORD	12026.000	YCOORD	10848.000
XCOORD	12120.000	YCOORD	10932.000
XCOORD	12214.000	YCOORD	11016.000
XCOORD	12308.000	YCOORD	11100.000
XCOORD	12402.000	YCOORD	11184.000
XCOORD	12496.000	YCOORD	11268.000
XCOORD	12590.000	YCOORD	11352.000
XCOORD	12684.000	YCOORD	11436.000
XCOORD	12778.000	YCOORD	11520.000
XCOORD	12872.000	YCOORD	11604.000
XCOORD	12966.000	YCOORD	11688.000
XCOORD	13060.000	YCOORD	11772.000
XCOORD	13154.000	YCOORD	11856.000
XCOORD	13248.000	YCOORD	11940.000
XCOORD	13342.000	YCOORD	12024.000
XCOORD	13436.000	YCOORD	12108.000
XCOORD	13530.000	YCOORD	12192.000
XCOORD	13624.000	YCOORD	12276.000
XCOORD	13718.000	YCOORD	12360.000
XCOORD	13812.000	YCOORD	12444.000
XCOORD	13906.000	YCOORD	12528.000
XCOORD	14000.000	YCOORD	12612.000
XCOORD	14094.000	YCOORD	12696.000
XCOORD	14188.000	YCOORD	12780.000
XCOORD	14282.000	YCOORD	12864.000
XCOORD	14376.000	YCOORD	12948.000
XCOORD	14470.000	YCOORD	13032.000
XCOORD	14564.000	YCOORD	13116.000
XCOORD	14658.000	YCOORD	13200.000
XCOORD	14752.000	YCOORD	13284.000
XCOORD	14846.000	YCOORD	13368.000
XCOORD	14940.000	YCOORD	13452.000
XCOORD	15034.000	YCOORD	13536.000
XCOORD	15128.000	YCOORD	13620.000
XCOORD	15222.000	YCOORD	13704.000
XCOORD	15316.000	YCOORD	13788.000
XCOORD	15410.000	YCOORD	13872.000
XCOORD	15504.000	YCOORD	13956.000
XCOORD	15598.000	YCOORD	14040.000
XCOORD	15692.000	YCOORD	14124.000
XCOORD	15786.000	YCOORD	14208.000
XCOORD	15880.000	YCOORD	14292.000
XCOORD	15974.000	YCOORD	14376.000
XCOORD	16068.000	YCOORD	14460.000
XCOORD	16162.000	YCOORD	14544.000
XCOORD	16256.000	YCOORD	14628.000
XCOORD	16350.000	YCOORD	14712.000
XCOORD	16444.000	YCOORD	14796.000
XCOORD	16538.000	YCOORD	14880.000
XCOORD	16632.000	YCOORD	14964.000
XCOORD	16726.000	YCOORD	15048.000
XCOORD	16820.000	YCOORD	15132.000
XCOORD	16914.000	YCOORD	15216.000
XCOORD	17008.000	YCOORD	15300.000
XCOORD	17102.000	YCOORD	15384.000
XCOORD	17196.000	YCOORD	15468.000
XCOORD	17290.000	YCOORD	15552.000
XCOORD	17384.000	YCOORD	15636.000
XCOORD	17478.000	YCOORD	15720.000
XCOORD	17572.000	YCOORD	15804.000
XCOORD	17666.000	YCOORD	15888.000
XCOORD	17760.000	YCOORD	15972.000
XCOORD	17854.000	YCOORD	16056.000
XCOORD	17948.000	YCOORD	16140.000
XCOORD	18042.000	YCOORD	16224.000
XCOORD	18136.000	YCOORD	16308.000
XCOORD	18230.000	YCOORD	16392.000
XCOORD	18324.000	YCOORD	16476.000
XCOORD	18418.000	YCOORD	16560.000
XCOORD	18512.000	YCOORD	16644.000
XCOORD	18606.000	YCOORD	16728.000
XCOORD	18700.000	YCOORD	16812.000
XCOORD	18794.000	YCOORD	16896.000
XCOORD	18888.000	YCOORD	16980.000
XCOORD	18982.000	YCOORD	17064.000
XCOORD	19076.000	YCOORD	17148.000
XCOORD	19170.000	YCOORD	17232.000
XCOORD	19264.000	YCOORD	17316.000
XCOORD	19358.000	YCOORD	17400.000
XCOORD	19452.000	YCOORD	17484.000
XCOORD	19546.000	YCOORD	17568.000
XCOORD	19640.000	YCOORD	17652.000
XCOORD	19734.000	YCOORD	17736.000
XCOORD	19828.000	YCOORD	17820.000
XCOORD	19922.000	YCOORD	17904.000
XCOORD	20016.000	YCOORD	17988.000
XCOORD	20110.000	YCOORD	18072.000
XCOORD	20204.000	YCOORD	18156.000
XCOORD	20298.000	YCOORD	18240.000
XCOORD	20392.000	YCOORD	18324.000
XCOORD	20486.000	YCOORD	18408.000
XCOORD	20580.000	YCOORD	

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 BOUNDARY CONDITIONS

RESTRICTED X-DEFLECTION = 1
 RESTRICTED Y-DEFLECTION = 0

C.L. BOTTOM
 BC SUPPORTS 1.000
 WEIGHT 1.000 0.000

THERE ARE 10 C.L. SUPPORTS

C.L. SUPPORTS ARE DEFINED FOR ROWS
 1 2 3 4 5 6 22 23 24 25

SUPPORT AT BOTTOM IS LOCATED ON COL 15 (NODE14)

ROW NUMBERING SYSTEM

C	ROW	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25
1	0	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1
2	1	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2	2
3	2	3	3	3	3	3	3	3	3	3	3	3	3	3	3	3	3	3	3	3	3	3	3	3	3	3
4	3	4	4	4	4	4	4	4	4	4	4	4	4	4	4	4	4	4	4	4	4	4	4	4	4	4
5	4	5	5	5	5	0	0	0	5	5	5	0	0	0	5	5	0	0	0	0	0	5	5	5	5	5
6	5	6	6	6	0	0	0	0	6	6	6	0	0	0	6	6	0	0	0	0	0	0	6	6	6	6
7	6	7	7	7	0	0	0	0	7	7	7	0	0	0	7	7	0	0	0	0	0	0	7	7	7	7
8	7	8	8	8	0	0	0	0	8	8	8	0	0	0	8	8	0	0	0	0	0	0	8	8	8	8
9	8	9	9	9	0	0	0	0	9	9	9	0	0	0	9	9	0	0	0	0	0	0	9	9	9	9
10	9	10	10	10	0	0	0	0	10	10	10	0	0	0	10	10	0	0	0	0	0	0	10	10	10	10
11	10	11	11	11	6	0	0	0	11	11	11	0	0	0	11	11	0	0	0	0	0	6	11	11	11	11
12	11	12	12	12	7	5	5	5	12	12	12	5	5	5	12	12	5	5	5	5	7	12	12	12	12	
13	12	13	13	13	8	6	6	6	13	13	13	6	6	6	13	13	6	6	6	6	8	13	13	13	13	
14	13	14	14	14	9	7	7	7	14	14	14	7	7	7	14	14	7	7	7	7	9	14	14	14	14	
15	14	15	15	15	10	8	8	8	15	15	15	8	8	8	15	15	8	8	8	8	10	15	15	15	15	
16	15	16	16	16	11	9	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	16	16	16	16	
17	16	17	17	17	12	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	17	17	17	
18	17	18	18	18	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	18	18	18	
19	18	19	19	19	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	19	19	19	
20	19	20	20	20	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	20	20	20	
21	20	21	21	21	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	21	21	21	
22	21	22	22	22	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	22	22	22	
23	22	23	23	23	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	23	23	23	
24	23	24	24	24	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	24	24	24	
25	24	25	25	25	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	25	25	25	
26	25	26	26	26	13	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	26	26	26	
27	26	27	27	27	14	10	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	17	27	27	27	

DEFINITION OF PLATE THICKNESSES

PLATE	THICKNESS	START ROW	END ROW	START COL	END COL	PLATE NO.
PLATE	1.200	22.	25.	1.	15.	(1)
PLATE	1.200	23.	25.	15.	27.	(2)
PLATE	1.400	11.	22.	1.	4.	(3)
PLATE	1.400	11.	22.	12.	15.	(4)
PLATE	1.600	4.	11.	1.	4.	(5)
PLATE	1.600	4.	11.	12.	15.	(6)
PLATE	1.800	1.	4.	1.	8.	(7)
PLATE	1.400	1.	4.	8.	15.	(8)
PLATE	2.350	1.	4.	15.	21.	(9)
PLATE	1.600	1.	4.	21.	27.	(10)
PLATE	2.100	4.	6.	4.	12.	(11)
PLATE	1.400	8.	15.	4.	12.	(12)
PLATE	1.500	15.	16.	4.	12.	(13)
PLATE	1.400	16.	22.	4.	12.	(14)
PLATE	1.400	21.	23.	15.	17.	(15)
PLATE	1.200	22.	23.	26.	27.	(16)
PLATE	2.100	4.	7.	15.	18.	(17)
PLATE	1.400	4.	6.	25.	27.	(18)
END	-0.000	-0.	-0.	-0.	-0.	(19)

NO. OF AREAS OF COMMON THICKNESSES (50) =18

DEFINITION OF PLATE THICKNESSES

C	ROW	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25	
0	1	0	7	7	7	5	5	5	5	5	5	5	3	3	3	3	3	3	3	3	3	3	3	1	1	1	
L	2	7	7	7	7	5	5	5	5	5	5	5	3	3	3	3	3	3	3	3	3	3	3	1	1	1	
	3	7	7	7	7	5	5	5	5	5	5	5	3	3	3	3	3	3	3	3	3	3	3	1	1	1	
	4	7	7	7	11	11	11	5	12	12	12	12	12	12	12	13	14	14	14	14	14	14	14	1	1	1	
	5	7	7	7	11	11	0	0	0	12	12	12	0	0	0	13	14	0	0	0	0	14	14	1	1	1	
	6	7	7	7	11	0	0	0	0	12	12	12	0	0	0	13	14	0	0	0	0	0	14	1	1	1	
	7	7	7	7	11	0	0	0	0	12	12	12	0	0	0	13	14	0	0	0	0	0	14	1	1	1	
	8	8	8	8	11	0	0	0	0	12	12	12	0	0	0	13	14	0	0	0	0	0	14	1	1	1	
	9	8	8	8	11	0	0	0	0	12	12	12	0	0	0	13	14	0	0	0	0	0	14	1	1	1	
	10	8	8	8	11	0	0	0	0	12	12	12	0	0	0	13	14	0	0	0	0	0	14	1	1	1	
	11	8	8	8	11	11	0	0	0	12	12	12	0	0	0	13	14	0	0	0	0	14	14	1	1	1	
	12	8	8	8	11	11	11	6	12	12	12	12	12	12	13	14	14	14	14	14	14	14	14	1	1	1	
	13	8	8	8	8	6	6	6	6	6	6	6	4	4	4	4	4	4	4	4	4	4	4	1	1	1	
	14	8	8	8	8	6	6	6	6	6	6	6	4	4	4	4	4	4	4	4	4	4	4	1	1	1	
	15	9	9	9	17	17	17	17	6	6	6	6	4	4	4	4	4	4	4	4	4	4	15	15	15	2	2
	16	9	9	9	17	17	17	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	15	15	2	2
	17	9	9	9	17	17	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	15	2	2	2
	18	9	9	9	17	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2	2	2
	19	9	9	9	9	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2	2	2
	20	9	9	9	9	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2	2	2
	21	10	10	10	10	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2	2	2
	22	10	10	10	10	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2	2	2
	23	10	10	10	10	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2	2	2
	24	10	10	10	10	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2	2	2
	25	10	10	10	18	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2	2	2
	26	10	10	10	18	18	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	16	2	2	2
	27	10	10	10	18	18	18	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	16	16	2	2

DEFINITION OF BAR ELEMENTS

	AX	START ROW	END ROW	START COL	END COL	
BAR	1257.830	1.	1.	2.	27.	(1)
BAR	1257.830	1.	2.	2.	1.	(2)
BAR	1257.830	2.	4.	1.	1.	(3)
BAR	1232.160	4.	24.	1.	1.	(4)
BAR	1309.170	25.	25.	1.	27.	(5)
BAR	135.000	1.	25.	4.	4.	(6)
BAR	135.000	1.	25.	12.	12.	(7)
BAR	40.000	23.	23.	15.	27.	(8)
BAR	40.000	22.	22.	1.	15.	(9)
BAR	718.760	22.	25.	8.	8.	(10)
BAR	718.760	23.	25.	21.	21.	(11)
BAR	795.770	1.	4.	8.	8.	(12)
BAR	795.770	1.	4.	21.	21.	(13)
BAR	130.000	9.	9.	4.	12.	(14)
BAR	130.000	10.	10.	4.	12.	(15)
BAR	130.000	11.	11.	4.	12.	(16)
BAR	140.000	15.	15.	4.	12.	(17)
BAR	140.000	16.	16.	4.	12.	(18)
BAR	63.250	9.	11.	7.	7.	(19)
BAR	63.250	9.	11.	9.	9.	(20)
BAR	63.250	15.	16.	7.	7.	(21)
BAR	63.250	15.	16.	9.	9.	(22)
BAR	75.000	4.	4.	1.	15.	(23)
BAR	744.430	1.	6.	27.	27.	(24)
BAR	744.430	22.	25.	27.	27.	(25)
BAR	24.000	20.	22.	4.	6.	(26)
BAR	24.000	20.	22.	12.	10.	(27)
BAR	24.000	21.	23.	15.	17.	(28)
BAR	18.000	21.	22.	4.	5.	(29)
BAR	18.000	21.	22.	12.	11.	(30)
BAR	18.000	22.	23.	15.	16.	(31)
BAR	18.000	22.	23.	27.	26.	(32)
BAR	18.000	4.	5.	5.	4.	(33)
BAR	18.000	4.	5.	11.	12.	(34)
BAR	18.000	4.	5.	26.	27.	(35)
BAR	75.000	4.	6.	6.	4.	(36)
BAR	75.000	4.	6.	10.	12.	(37)
BAR	75.000	4.	7.	18.	15.	(38)
BAR	21.600	4.	6.	17.	15.	(39)
BAR	50.000	4.	6.	25.	27.	(40)
BAR	273.000	4.	4.	15.	27.	(41)
BAR	75.000	1.	4.	18.	18.	(42)
BAR	75.000	1.	4.	24.	24.	(43)
BAR	70.000	23.	25.	18.	18.	(44)
BAR	70.000	23.	25.	24.	24.	(45)
BAR	75.000	7.	7.	1.	4.	(46)
BAR	75.000	7.	7.	12.	15.	(47)
BAR	75.000	13.	13.	1.	4.	(48)
BAR	75.000	13.	13.	12.	15.	(49)
BAR	75.000	19.	19.	1.	4.	(50)
BAR	75.000	19.	19.	12.	15.	(51)
BAR	75.000	9.	9.	1.	3.	(52)
BAR	75.000	9.	9.	13.	15.	(53)
BAR	75.000	10.	10.	1.	3.	(54)
BAR	75.000	10.	10.	13.	15.	(55)
BAR	75.000	11.	11.	1.	3.	(56)
BAR	75.000	11.	11.	13.	15.	(57)
BAR	75.000	15.	15.	1.	3.	(58)
BAR	75.000	15.	15.	13.	15.	(59)
BAR	21.600	1.	4.	5.	5.	(60)
BAR	75.000	16.	16.	1.	3.	(61)
BAR	75.000	16.	16.	13.	15.	(62)
END	-0.000	-0.	-0.	-0.	-0.	(63)

NO. BAR ELEMENTS (100) 62

DEFINITION OF LONGITUDINALS

STANDA	I	AX	ROW	STRT COL	END COL
	IX	IY	(COL)	(STRT ROW)	(END ROW)
XLONG	.23000E+04	.30000E+06	1.	22.	26.
XLONG	.23000E+04	.35000E+06	1.	16.	20.
XLONG	.23000E+04	.41000E+06	1.	12.	14.
XLONG	.23000E+04	.45000E+06	1.	9.	11.
XLONG	.23000E+04	.53000E+06	1.	6.	7.
XLONG	.23000E+04	.59000E+06	1.	4.	5.
XLONG	.23000E+04	.61000E+06	1.	2.	3.
XLONG	.12300E+06	.32800E+08	1.	27.	27.
XLONG	.36400E+05	.95200E+07	1.	21.	21.
XLONG	.33300E+05	.92100E+07	1.	8.	8.
XLONG	.10000E+02	.66462E+05	25.	2.	7.
XLONG	.10000E+02	.66462E+05	25.	9.	14.
XLONG	.10000E+02	.66462E+05	25.	16.	20.
XLONG	.10000E+02	.66462E+05	25.	22.	26.
XLONG	.45000E+04	.79300E+07	25.	27.	27.
XLONG	.26000E+04	.32870E+07	25.	21.	21.
XLONG	.26000E+04	.43700E+07	25.	8.	8.
YLONG	.23500E+06	.30000E+04	1.	2.	2.
YLONG	.21000E+06	.30000E+04	1.	3.	4.
YLONG	.19000E+06	.30000E+04	1.	5.	5.
YLONG	.15500E+06	.30000E+04	1.	6.	6.
YLONG	.23200E+06	.30000E+04	1.	7.	8.
YLONG	.15500E+06	.30000E+04	1.	9.	10.
YLONG	.12650E+06	.30000E+04	1.	11.	11.
YLONG	.18900E+06	.30000E+04	1.	12.	12.
YLONG	.37900E+06	.30000E+04	1.	13.	13.
YLONG	.21000E+06	.30000E+04	1.	14.	14.
YLONG	.90000E+05	.30000E+04	1.	15.	16.
YLONG	.84000E+05	.30000E+04	1.	17.	18.
YLONG	.60000E+05	.30000E+04	1.	19.	20.
YLONG	.21000E+05	.20000E+04	1.	21.	23.
YLONG	.32000E+05	.20000E+04	15.	2.	2.
YLONG	.14500E+06	.20000E+04	15.	3.	4.
YLONG	.15800E+06	.20000E+04	15.	5.	6.
YLONG	.23700E+06	.20000E+04	15.	7.	8.
YLONG	.15800E+06	.20000E+04	15.	9.	10.
YLONG	.90000E+05	.20000E+04	15.	11.	11.
YLONG	.13500E+06	.20000E+04	15.	12.	12.
YLONG	.19000E+06	.20000E+04	15.	13.	13.
YLONG	.10000E+06	.20000E+04	15.	14.	14.
YLONG	.50000E+05	.20000E+04	15.	15.	16.
YLONG	.70000E+05	.20000E+04	15.	17.	18.
YLONG	.50000E+05	.20000E+04	15.	19.	20.
YLONG	.21000E+05	.20000E+04	15.	21.	23.
YLONG	.15000E+05	.10000E+04	15.	24.	24.
END	-0.	-0.	-0.	-0.	-0.

**THERE ARE A TOTAL OF 95 LONGITUDINALS

LONGITUDINAL NUMBERING SYSTEM

C	ROW	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25
L	1	27	54	81	108	122	132	140	148	163	178	193	201	209	217	232	247	255	263	271	279	289	306	0	0	
	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0361
	2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0362
	3	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0363
	4	0	0	0	0	1	1	1	0	0	0	1	1	1	0	0	1	1	1	1	1	0	0	0	0	0364
	5	0	0	0	1	1	1	1	0	0	0	1	1	1	0	0	1	1	1	1	1	1	0	0	0	0365
	6	0	0	0	1	1	1	1	0	0	0	1	1	1	0	0	1	1	1	1	1	1	0	0	0	0366
	7	0	0	0	1	1	1	1	0	0	0	1	1	1	0	0	1	1	1	1	1	1	0	0	0	0367
	8	0	0	0	1	1	1	1	0	0	0	1	1	1	0	0	1	1	1	1	1	1	0	0	0	0368
	9	0	0	0	1	1	1	1	0	0	0	1	1	1	0	0	1	1	1	1	1	1	0	0	0	0369
	10	0	0	0	0	1	1	1	0	0	0	1	1	1	0	0	1	1	1	1	1	0	0	0	0	0370
	11	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0371
	12	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0372
	13	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0373
	0	41	68	95	117	129	139	147	162	177	192	200	208	216	231	246	254	262	270	278	288	303	332	0347	0	
	15	0	0	0	0	0	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	0	0	0375
	16	0	0	0	0	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	0	0376
	17	0	0	0	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	0	0377
	18	0	0	0	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	0	0378
	19	0	0	0	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	0	0379
	20	0	0	0	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	0	0380
	21	0	0	0	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	0	0381
	22	0	0	0	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	0	0382
	23	0	0	0	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	0	0383
	24	0	0	0	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	0	0384
	25	0	0	0	0	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	0	0385
	26	0	0	0	0	0	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	0	0	0386

LOADING CONDITION

YUNIFO	0.000	135.773	3080.400	-103.428	6160.800	135.773
YUNIFO	10781.400	-101.764				

XLONG	1.	16.	26.	1.000		
YUNIFO	0.000	-98.728	3080.400	135.773	6160.800	-98.703
YUNIFO	10781.400	-9.969				

XLONG	1.	2.	14.	1.000		
XUNIFO	0.000	-92.680	3080.400	129.990	6160.800	-92.680
XUNIFO	10781.400	-4.640				

XLONG	1.	2.	2.	1.000		
XUNIFO	0.000	-92.680	3080.400	129.990	6160.800	-92.680
XUNIFO	10781.400	-4.640				

YLONG	1.	2.	2.	1.000		
XUNIFO	0.000	-84.210	3080.400	129.990	6160.800	-84.210
XUNIFO	10781.400	3.430				

YLONG	1.	3.	3.	1.000		
XUNIFO	0.000	-70.940	3081.400	121.330	6160.800	-70.940
XUNIFO	10781.400	10.850				

YLONG	1.	4.	4.	1.000		
XUNIFO	0.000	-63.570	3081.400	121.330	6160.800	-63.570
XUNIFO	10781.400	16.730				

YLONG	1.	5.	5.	1.000		
XUNIFO	0.000	-56.570	3081.400	121.330	6160.800	-56.570
XUNIFO	10781.400	22.600				

YLONG	1.	6.	6.	1.000		
XUNIFO	0.000	-52.000	3081.400	118.283	6160.800	-52.000
XUNIFO	10781.400	28.470				

YLONG	1.	7.	7.	1.000		
XUNIFO	0.000	-52.000	3081.400	118.283	6160.800	-52.000
XUNIFO	10781.400	28.470				

YLONG	1.	8.	8.	1.000		
XUNIFO	0.000	-48.058	3081.330	100.433	6160.800	-48.050
XUNIFO	10781.400	33.750				

YLONG	1.	9.	9.	1.000		
XUNIFO	0.000	-48.050	3081.400	100.433	6160.800	-48.050
XUNIFO	10781.400	33.750				

YLONG	1.	10.	10.	1.000		
XUNIFO	0.000	-48.050	3081.400	93.233	6160.800	-48.050
XUNIFO	10781.400	33.750				

YLONG	1.	11.	11.	1.000		
XUNIFO	10781.400	33.750				
XUNIFO	0.000	-48.050				

YLONG	1.	12.	12.	1.000		
XUNIFO	0.000	-96.090	3081.400	92.783	6160.800	-96.090
XUNIFO	10781.400	67.500				

YLONG	1.	13.	13.	1.000		
XUNIFO	0.000	-96.090	3081.400	92.783	6160.800	-96.090
XUNIFO	10781.400	67.500				

YLONG	1.	14.	14.	1.000		
XUNIFO	0.000	-48.050	3081.400	35.583	6160.800	-48.050
XUNIFO	10781.400	33.750				

YLONG	1.	15.	15.	1.000		
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YLONG	1.	16.	16.	1.00
XUNIFO	0.000	-48.050	3081.400	21.17
XUNIFO	10781.400	22.490		
YLONG	1.	17.	17.	1.00
XUNIFO	0.000	-96.090	3081.400	13.96
XUNIFO	10781.400	14.990		
YLONG	1.	18.	18.	1.00
XUNIFO	0.000	-54.750	3081.400	6.76
XUNIFO	10781.400	7.480		
YLONG	1.	19.	19.	1.00
XUNIFO	0.000	-88.700	3081.400	-0.00
XUNIFO	10781.400	.020		
YLONG	1.	20.	20.	1.00
XUNIFO	0.000	-33.940	3081.400	-0.00
YLONG	1.	21.	21.	1.00
XUNIFO	0.000	-27.000	3081.400	-0.00
YLONG	1.	22.	22.	1.00
XUNIFO	0.000	-37.760	3081.400	-0.00
YLONG	1.	23.	23.	1.00
XUNIFO	0.000	223.000	3080.400	-223.00
XUNIFO	10781.400	-88.150		
YLONG	15.	2.	2.	1.00
XUNIFO	0.000	215.030	3080.400	-215.03
XUNIFO	10781.400	-88.150		
YLONG	15.	3.	3.	1.00
XUNIFO	0.000	193.510	3080.400	-193.51
XUNIFO	10781.400	-82.280		
YLONG	15.	4.	4.	1.00
XUNIFO	0.000	186.570	3080.400	-186.57
XUNIFO	10781.400	-82.280		
YLONG	15.	5.	5.	1.00
XUNIFO	0.000	179.630	3080.400	-179.63
XUNIFO	10781.400	-82.280		
YLONG	15.	6.	6.	1.00
XUNIFO	0.000	172.690	3080.400	-172.69
XUNIFO	10781.400	-82.280		
YLONG	15.	7.	7.	1.00
XUNIFO	0.000	172.690	3080.400	-172.69
XUNIFO	10781.400	-82.280		
YLONG	15.	8.	8.	1.00
XUNIFO	0.000	172.690	3080.400	-172.69
XUNIFO	10781.400	-82.280		

YLONG	15.	11.	11.	1.000		
XUNIFO	0.000	131.070	3080.400	-131.070	6160.800	131.070
XUNIFO	10781.400	-82.280				
YLONG	15.	12.	12.	1.000		
XUNIFO	0.000	199.700	3080.400	-199.700	6160.800	199.700
XUNIFO	10781.400	-164.550				
YLONG	15.	13.	13.	1.000		
XUNIFO	0.000	199.700	3080.400	-199.700	6160.800	199.700
XUNIFO	10781.400	-164.550				
YLONG	15.	14.	14.	1.000		
XUNIFO	0.000	89.450	3080.400	-89.450	6160.800	89.450
XUNIFO	10781.400	-82.280				
YLONG	15.	15.	15.	1.000		
XUNIFO	0.000	82.500	3080.400	-82.500	6160.800	82.500
XUNIFO	10781.400	-82.500				
YLONG	15.	16.	16.	1.000		
XUNIFO	0.000	75.570	3080.400	-75.570	6160.800	75.570
XUNIFO	10781.400	-75.570				
YLONG	15.	17.	17.	1.000		
XUNIFO	0.000	130.330	3080.400	-130.330	6160.800	130.330
XUNIFO	10781.400	-103.330				
YLONG	15.	18.	18.	1.000		
XUNIFO	0.000	54.750	3080.400	-54.750	6160.800	54.750
XUNIFO	10781.400	-54.750				
YLONG	15.	19.	19.	1.000		
XUNIFO	0.000	88.700	3080.400	-88.700	6160.800	88.700
XUNIFO	10781.400	-88.700				
YLONG	15.	20.	20.	1.000		
XUNIFO	0.000	33.940	3080.400	-33.940	6160.800	33.940
XUNIFO	10781.400	-33.940				
YLONG	15.	21.	21.	1.000		
XUNIFO	10781.400	-37.000				
XUNIFO	0.000	37.000				
YLONG	15.	22.	22.	1.000		
XUNIFO	0.000	39.360	3080.400	-39.360	6160.800	39.360
XUNIFO	10781.400	-39.360				
YLONG	15.	23.	23.	1.000		
XUNIFO	0.000	9.840	3080.400	-0.000	6160.800	9.840
XUNIFO						
XLONG	25.	2.	2.	1.000		
XUNIFO	0.000	9.750	3080.400	-0.000	6160.800	9.750
XUNIFO						
XLONG	25.	3.	3.	1.000		
XUNIFO	0.000	8.970	3080.400	-0.000	6160.800	8.970
XUNIFO						
XLONG	25.	4.	4.	1.000		
XUNIFO	0.000	8.190	3080.400	-0.000	6160.800	8.190
XUNIFO						
XLONG	25.	5.	5.	1.000		
END						

SHEAR LOADS ON TRANSVERSES			
	TRANSV.	SHELL	BMD
	SHEAR	SHEAR	SHEAR
SHEAR	.10000E+01	-.85715E+05	.25537E+06
SHEAR	.20000E+01	.43520E+04	-.14017E+06
SHEAR	.30000E+01	.20857E+06	-.58370E+05
SHEAR	.40000E+01	.73500E+04	.14527E+06
SHEAR	.50000E+01	-.93507E+05	.23078E+06
SHEAR	.60000E+01	.16439E+06	.78511E+05
SHEAR	.70000E+01	.33053E+06	.18267E+05
SHEAR	.80000E+01	.23506E+06	.97579E+05
SHEAR	.90000E+01	.97556E+05	.24047E+06
SHEAR	.10000E+02	.23501E+06	.97625E+05
SHEAR	.11000E+02	.33049E+06	.18337E+05
SHEAR	.12000E+02	.16409E+06	.78563E+05
SHEAR	.13000E+02	-.93485E+05	.23007E+06
SHEAR	.14000E+02	.73120E+04	.14519E+06
SHEAR	.15000E+02	.21831E+06	-.70167E+05
SHEAR	.16000E+02	.13206E+05	.13645E+06
SHEAR	.17000E+02	.13016E+06	.13495E+06
SHEAR	.18000E+02	.22457E+06	-.71362E+05
SHEAR	.19000E+02	.11510E+04	.13279E+06
SHEAR	.20000E+02	-.55349E+05	.26110E+06
SHEAR	.21000E+02	-.12921E+06	-.14439E+06
SHEAR	.22000E+02	-.26237E+06	-.49057E+06
SHEAR	.23000E+02	-.31564E+06	-.36653E+06
SHEAR	.24000E+02	-.49983E+06	-.20042E+06
SHEAR	.25000E+02	-.32807E+06	-.36763E+06
SHEAR	.26000E+02	-.32784E+06	-.36697E+06
SHEAR	.27000E+02	-.50378E+06	-.19915E+06
SHEAR	.28000E+02	-.31189E+06	-.35945E+06
SHEAR	.29000E+02	-.29217E+06	-.49742E+06
END	-0.	-0.	-0.

Program Execution Times

Execution time varies according to the structural and loading definitions.

The table below provides sample computer times for the CDC6600 and the UNIVAC 1108 (EXEC II) computers.

Table 1. CDC 6600 Program Execution Times

	No. Transverses	No. Frame Elements	No. Longitudinals	No. Equivalent Elements	No. Eigenvalues Used	CP Seconds	System Seconds
Complete Analysis	29	618	95	20,772	1	129.656	975.
" "	28	699	95	22,327	1	129.052	1038.
" "	29	699	95	23,121	1	129.029	1043.
" "	3	66	"	242	1	14,409	72.
Stripped Package *	3	36	"	141	1	3.403	20.
CDC/EASE **	-Quarter Model-		"	92	N.A.	10.977	41.
Data Check	28	699	94	22,298	-0-***	16.008	25/

* This stripped package does not have input routines amenable for convenient use of the transverse analysis capabilities.

** Since the problem analyzed was symmetrical fore-aft as well as about the hull centerline, the CDC/EASE Analysis could be simplified to a quarter-hull model. The tanker transverse program, on the other hand analyzed the whole half-section. Nevertheless, note the significant difference in computer times between the two analyses. The additional computer time required for execution of the complete tanker program (as opposed to the stripped version) is mainly due to the extensive input processing which greatly facilitates the problem definition.

*** If the EIGENS card is omitted, the program will function in data checking only. This procedure is recommended before a complete stress analysis is attempted.

Table 2. UNIVAC 1108 (Exec II) Program Execution Times

	No. Transverses	No. Frame Elements	No. Longitudinals	No. Equivalent Elements	No. Eigenvalues Used	CP Seconds	System Seconds
Complete Analysis	29	444	77	15,186	1	363	N/A

OUTPUT DESCRIPTION:

The plate and bar element stresses are given along the locally defined element axes as established by the program's automatic element generation. Figure 6 below illustrates the local axes for each type of element:

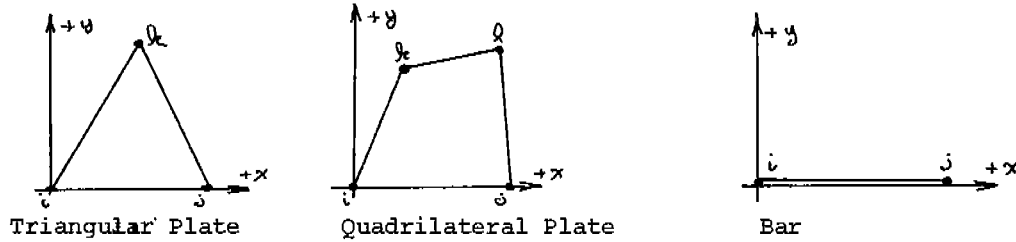


Fig. 6. Local Coordinate System for Finite Elements

The positive local x-direction extends from node i to j for all elements. The positive y-direction is always 90° counter-clockwise.

The following table as generated by the program provides the row, node coordinates for each element node i,j,k, and l as appropriate. Note that the node number given corresponds to the column number except that where void nodes exist on the given row, the node number is a re-sequenced column number with **all** void nodes omitted from the count from the global x=0.

Table 3.

MEMBER DATA			OUTPUT VARIABLES							i		j		k		l		PLATE THICKNESS OF BAR X-SECTIONAL AREA P1
M	M	I	I	I	I	I	I	EOR	JOR	EOR	JOR	EOR	JOR	EOR	JOR			
M	M	I	I	I	I	I	I	I	J	I	J	I	J	I	J	1.80000		
F	F	E	F	F	F	F	F	N	N	N	N	N	N	N	N	1257.83000		
M	M	G	S	T	J	K	L	I	I	J	J	K	K	L	L	1.80000		
N	T	N	F													1.80000		
O	Y	U														1.80000		
	P															1.80000		
1	1	0	2	0	0	0	0	1	1	2	1	2	2	0	0	1.80000		
2	5	0	2	0	0	0	0	1	1	2	1	0	0	0	0	1257.83000		
3	2	0	2	0	0	0	0	1	1	1	2	2	2	2	3	1.80000		
4	5	0	2	0	0	0	0	1	1	1	2	0	0	0	0	1257.83000		
5	2	0	2	0	0	0	0	1	2	1	3	2	3	2	4	1.80000		
6	5	0	2	0	0	0	0	1	2	1	3	0	0	0	0	1257.83000		
7	2	0	2	0	0	0	0	1	3	1	4	2	4	2	5	1.80000		
8	5	0	2	0	0	0	0	1	3	2	4	0	0	0	0	135.00000		
9	5	0	2	0	0	0	0	1	3	1	4	0	0	0	0	1257.83000		
10	2	0	2	0	1	0	1	1	4	1	5	2	5	2	6	1.80000		

MEMTYP indicates the type of element generated by the program:
 1 = triangular plate; 2 = quadrilateral plate; and 5 = bar element.
 The member numbers are relative to the given row only.

Table 4 lists the stress solutions for the sample web frame elements defined in Table 3. The x-stress, for the bar element, is always directed along the axis of the bar (from node i to node j).

The quadrilateral plate stresses are always given in the same directional (x-y) sense as the global coordinate axes of the web frame, which is basically an orthogonal, rectangular grid network. Stresses, however, may be interpolated to locations within the quadrilateral element by using the following equations:

$$\begin{aligned}\sigma_x &= \sigma_{xi} + \gamma_x \cdot y \\ \sigma_y &= \sigma_{yi} + \gamma_y \cdot x\end{aligned}$$

where x and y are the relative distances of the interior point in the element from the ith node (lower left hand corner of the element).

Stresses within the triangular plate element, however, are somewhat more difficult to transpose to the coordinate system of the web frame. The stress, assumed constant for this type of element, may be computed from the following equation:

$$\sigma = \sigma_x \cos^2\alpha + \sigma_y \sin^2\alpha + 2\tau_{xy} \sin\alpha \cos\alpha$$

where α is the angle between the x-axis of the triangular element (see Figure 6) and the x-axis of the web frame (horizontal and directed left-to-right).

Table 4.

MEMBER STRESSES IN	K	G	PER SQUARE	σ_x	σ_y	τ_{xy}	1ST PRINC STR	2ND PRINC STR	ANGLE 1ST PRINC STRS TO X-AXIS
LOAD SYSTEM	PNM	MEMBER TYPE AND NUMBER	MEMBER	X-STRESS (TRIANG PLATE)	Y-STRESS (QUAD PLATE)	SHEAR STRESS	1ST PRINC STR	2ND PRINC STR	ANGLE 1ST PRINC STRS TO X-AXIS
				σ_x	σ_y	τ_{xy}	σ_1	σ_2	
1	1	TRIANG PLATE	1	-.866627E+03	-.272493E+04	.983988E+03	-.642429E+03	-.314912E+04	23.3209 DEG
2	1	BAR	2	-.491491E+02					
3	1	QUAD PLATE	3	-.283891E+03	.101335E+02	-.247101E+04	.361794E+02	-.160762E+03	
4	1	BAR	4	-.472798E+02					
5	1	QUAD PLATE	5	.122356E+03	.282870E+00	.882559E+03	-.690638E+01	.196259E+02	
6	1	BAR	6	-.450321E+02					
7	1	QUAD PLATE	7	.705261E+02	.331108E+00	.218462E+03	.146913E+01	-.407095E+03	
8	1	BAR	8	.192834E+03					
9	1	BAR	9	-.157270E+02					
10	1	QUAD PLATE	10	.142271E+03	-.666457E+00	.364416E+03	-.842156E+00	-.532266E+03	
11	1	BAR	11	.330932E+03					
12	1	BAR	12	.447402E+02					
13	1	QUAD PLATE	13	.199936E+03	-.119272E+01	.295648E+03	-.400055E+00	-.580134E+03	
14	1	BAR	14	.251769E+03					
15	1	BAR	15	.116882E+03					
16	1	QUAD PLATE	16	.238587E+03	-.161659E+01	.263916E+03	-.197141E+01	-.490311E+03	
17	1	BAR	17	.214164E+03					
18	1	BAR	18	.187209E+03					
19	1	QUAD PLATE	19	.300920E+03	-.199119E+01	.922462E+02	.193693E+01	-.516085E+03	
20	1	BAR	20	.286513E+02					
21	1	BAR	21	.247345E+03					
22	1	QUAD PLATE	22	.364086E+03	-.344753E+01	.264207E+03	-.115805E+00	-.250201E+03	
23	1	BAR	23	.201523E+03					
24	1	BAR	24	.286456E+03					
25	1	QUAD PLATE	25	.383513E+03	-.379952E+01	.254398E+03	.426107E+00	-.398679E+02	

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